

CAPÍTULO 6

MEMORIA DE CÁLCULO

6.1. DATOS GENERALES

Destino: Edificio de educación Universitaria, Ampliación de la Facultad Regional San Rafael, Universidad Tecnológica Nacional.

Ubicación: Av. Urquiza 314, San Rafael, Mendoza.

Materiales de Construcción: Hormigón armado, acero y mampostería.

6.2. ESTRUCTURACIÓN

Sistema de cargas verticales y laterales: Pórticos de hormigón armado, tabiques de hormigón y tabiques de mampostería encadenada.

Sistemas de piso: Losa de hormigón armado tipo alivianada, con viguetas pretensadas.

Sistema de cubierta superior: Cubierta liviana de chapa galvanizada trapezoidal N°22, estructura de perfiles de chapa tipo “hoesch” apoyados sobre cerchas metálicas de perfiles normales tipo “L” con diagonales y montantes de ADN420s soldados.

6.3. PROPIEDADES DE LOS MATERIALES

6.3.1. HORMIGÓN ARMADO

- Hormigón Estructural H20 según especificaciones de CIRSOC 201
- Resistencia a la Compresión: $f'c = 200 \text{ kg/cm}^2$
- Módulo de Elasticidad Longitudinal: $E = 215000 \text{ kg/cm}^2$
- Tamaño máximo del agregado grueso: 25.4mm
- Cemento Pórtland Común que cumpla con IRAM 50000-2000 y CIRSOC 201.
- Contenido de cemento: 300 kg/m^3
- Relación agua/cemento: $a/c = 0.60$
- Dosificación: 1:5 (cemento : agregado)

6.3.2. ACERO DE REFUERZO

- Acero de Dureza Natural ADN420s soldados
- Tensión de Fluencia: $f_y = 4200 \text{ kg/cm}^2$
- Módulo de Elasticidad Longitudinal: $E = 2100000 \text{ kg/cm}^2$
- Responde a las Normas: CIRSOC 201 e IRAM-IAS U500-207
- Nervuradas
- Anclajes y empalmes $\geq 30 \text{ cm}$
- Recubrimiento mínimo 2cm

- Recubrimiento mínimo en bases 5cm

6.3.3. ACERO ESTRUCTURAL

- Acero tipo F-22
- Tensión de fluencia 2200 kg/cm²
- Módulo de Elasticidad Longitudinal: E = 2100000 kg/cm²
- $\gamma = 1.60$
- Tensión admisible: 1250 kg/cm²

6.3.4. MAMPOSTERÍA

- Mampostería de Ladrillos Cerámicos Macizos (LCM) según CIRSOC 103 parte III
- Resistencia a la compresión $f'_{cu} = 5 \text{ kg/cm}^2$
- Mortero tipo E, resistencia mínima a compresión a 28 días, 150 kg/cm²
- Docificación 1 : 0,25 : 3 (cemento:cal:arena)
- Resistencia al corte 2 kg/cm²
- Módulo de Elasticidad Longitudinal: E = 24000 kg/cm²
- Módulo de Corte: G = 4800 kg/cm²

6.3.5. TERRENO NATURAL

- Suelo tipo 1 (CIRSOC 103) sitio SC
- Resistencia a la compresión 2 kg/cm², las especificaciones técnicas de la edificación existente señala esta resistencia, ver ANEXO N°1.
- Todo elemento estructural en contacto con el suelo, llevará 5cm de hormigón de limpieza

6.3.6. ANCLAJE QUÍMICO

- Adhesivo Epoxi de dos componentes para anclajes
- Alta capacidad de carga
- Rápido Curado
- Se recomienda usar Sika® AnchorFix®-3001, especificaciones en el ANEXO 4.

6.3.7. MANGUITOS DE EMPALME

- Manguitos que respete las disposiciones del CIRSOC 201 respecto a empalmes mecánicos.
- Utilizar Sistema QUICK WEDGE® de NVENT LENTON®, especificaciones en el ANEXO 4.

6.3.8. UNIÓN HORMIGÓN ENDURECIDO – HORMIGÓN FRESCO

- Agente de unión epoxídico, tixotrópico, sin solventes.
- Utilizar Sikadur®-32 Gel, especificaciones en el ANEXO 4.

6.4. ANÁLISIS ESTRUCTURAL

El análisis estructural se encuentra en el *punto 5.3.2.6 “Análisis de Carga”* en el *capítulo 5*.

6.5. PLANILLAS DE CÁLCULO

6.5.1. ANÁLISIS SÍSMICO EN PLANTA BAJA

SISMO P.B. - BLOQUE 1		
FZA. SISMICA EN X	FZA. SISMICA EN Y	
Qx=Qy	248.20	Qx=Qy 248.20
Co	0.15	Co 0.15
γd	1.40	γd 1.40
γi	1.00	γi 1.00
γdux=γduy	1.3	γdux=γduy 1.3
S	0.80	S 0.80
Cx=Cy	0.2184	Cx=Cy 0.2184
Fsx	54.21	Fsy 54.21

SISMO VINCULADO EN PLANTA BAJA - BLOQUE 1																
F _x [t]:		54.21 CORTE EN NIVEL "X"		F _y [t]:		54.21 CORTE EN NIVEL "Y"										
E _x [m]:		0.07 L _x [m]:		18.20 M _{tx} max.:		253.15 M _{ty} max.:		151.96								
E _y [m]:		3.10 L _y [m]:		10.50 M _{tx} min.:		82.40 M _{ty} min.:		-144.01								
DIR.	PLANO "i"	D _i [t]	Q _p [t]	d _i [m]	Q _{mt} [t]				Q _t [t]	Corte en Muros						
					X		Y			L [cm]	b [cm]	Q _m [kg]	τ _m [kg/cm ²]	τ _m [adm]		
					max.	min.	max.	min.								
X	1	9.77	1.89	8.86	1.78	0.58	1.07	-1.01	3.67	-	-	3667	-	-	-	-
	2	42.19	8.15	5.76	5.00	1.63	3.00	-2.84	13.15	-	-	13152	-	-	-	-
	3	228.60	44.17	-1.44	-6.78	-2.21	-4.07	3.86	37.39	3600	20	37388	0.58	2.00	2.00	2.00
Y	4	57.85	16.69	9.00	10.71	3.49	6.43	-6.10	23.13	760	20	23125	1.69	2.00	2.00	2.00
	5	7.15	2.06	4.50	0.66	0.22	0.40	-0.38	2.46	-	-	2461	-	-	-	-
	6	57.85	16.69	0.00	0.00	0.00	0.00	0.00	16.69	760	20	16694	1.22	2.00	2.00	2.00
	7	7.15	2.06	4.50	0.66	0.22	0.40	-0.38	2.46	-	-	2461	-	-	-	-
	8	57.85	16.69	9.00	10.71	3.49	6.43	-6.10	23.13	760	20	23125	1.69	2.00	2.00	2.00

SISMO P.B. - BLOQUE 2		
FZA. SISMICA EN X	FZA. SISMICA EN Y	
Qx=Qy (t)	281.09	Qx=Qy (t) 281.09
Co	0.15	Co 0.15
γ_d	1.40	γ_d 1.40
γ_i	1.00	γ_i 1.00
$\gamma_{dux}=\gamma_{duy}$	1.3	$\gamma_{dux}=\gamma_{duy}$ 1.3
S	0.80	S 0.80
Cx=Cy	0.2184	Cx=Cy 0.2184
Fsx=Fsy	61.39	Fsx=Fsy 61.39

SISMO VINCULADO EN PLANTA BAJA - BLOQUE 2														
Fx[t]:		61.39		CORTE EN NIVEL "X"		Fy[t]:		61.39		CORTE EN NIVEL "Y"				
Ex[m]:	0.00	Lx[m]:	17.60	Mtxmax.:	320.12	Mtymin.:	162.07	Ex[m]:	0.00	Ly[m]:	10.50	Mtymin.:	-162.07	
Ey[m]:	3.64	Ly[m]:	10.50	Mtxmin.:	126.74	Mtymin.:	-162.07							
DIR.	PLANO "i"	Di [t]	Qp [t]	di [m]	Qmt [t]				Qt [t]	Corte en Muros				
					X		Y			L [cm]	b [cm]	Qm [kg]	τ_m [kg/cm ²]	τ_m [adm]
					max.	min.	max.	min.						
X	1	9.77	2.20	8.82	2.36	0.94	1.20	-1.20	4.56	-	-	4559	-	-
	2	42.19	9.50	5.72	6.62	2.62	3.35	-3.35	16.11	-	-	16113	-	-
	3	220.80	49.69	-1.48	-8.98	-3.55	-4.55	4.55	40.72	1760	20	40716	1.29	2.00
Y	9	57.85	18.91	8.70	13.81	5.47	6.99	-6.99	25.89	760	20	25895	1.89	2.00
	10	7.15	2.34	4.50	0.88	0.35	0.45	-0.45	2.78	-	-	2783	-	-
	11	57.85	18.91	0.00	0.00	0.00	0.00	0.00	18.91	-	-	18905	-	-
	12	7.15	2.34	4.50	0.88	0.35	0.45	-0.45	2.78	-	-	2783	-	-
	13	57.85	18.91	8.70	13.81	5.47	6.99	-6.99	25.89	760	20	25895	1.89	2.00

SISMO P.B. - BLOQUE 3		
FZA. SISMICA EN X	FZA. SISMICA EN Y	
Qx=Qy	336.30	336.30
Co	0.15	0.15
γd	1.40	1.40
γi	1.00	1.00
$\gamma dux = \gamma duy$	1.3	1.3
S	0.80	0.80
Cx=Cy	0.2184	0.2184
Fsx=Fsy	73.45	73.45

SISMO VINCULADO EN PLANTA BAJA - BLOQUE 3														
Fx [t]:		73.45	CORTE EN NIVEL "X"		Fy [t]:	73.45		CORTE EN NIVEL "Y"						
Ex [m]:		-0.08	Lx [m]:	27.20	Mtxmax.:	394.93	Mtxmax.:	293.77						
Ey [m]:		3.80	Ly [m]:	10.50	Mtymin.:	163.57	Mtymin.:	-305.57						
DIR.	PLANO "i"	Di [t]	Qp [t]	di [m]	Qmt [t]		Y		Qt [t]	Corte en Muros				
					max.	min.	max.	min.		L [cm]	b [cm]	Qm [kg]	τm [kg/cm ²]	τm [adm]
X	1	14.85	2.26	9.33	1.95	0.81	1.45	-1.51	4.21	-	-	4212	-	-
	2	44.05	6.70	6.23	3.87	1.60	2.88	-2.99	10.57	-	-	10567	-	-
	3	424.19	64.49	-0.97	-5.82	-2.41	-4.33	4.51	58.67	3600	20	58669	0.91	2.00
Y	14	57.85	16.80	13.50	11.02	4.56	8.20	-8.53	25.00	760	20	25002	1.83	2.00
	15	7.15	2.08	9.00	0.91	0.38	0.68	-0.70	2.75	-	-	2752	-	-
	16	57.85	16.80	4.50	3.67	1.52	2.73	-2.84	19.54	760	20	19537	1.43	2.00
	17	7.15	2.08	0.00	0.00	0.00	0.00	0.00	2.08	-	-	2077	-	-
	18	57.85	16.80	4.50	3.67	1.52	2.73	-2.84	19.54	760	20	19537	1.43	2.00
	19	7.15	2.08	0.00	0.91	0.38	0.68	-0.70	2.75	-	-	2752	-	-
	20	57.85	16.80	13.50	11.02	4.56	8.20	-8.53	25.00	760	20	25002	1.83	2.00

SISMO P.B. - BLOQUE 4		
FZA. SISMICA EN X	FZA. SISMICA EN Y	
Qx=Qy	197.33	Qx=Qy 197.33
Co	0.15	Co 0.15
γ_d	1.40	γ_d 1.40
γ_i	1.00	γ_i 1.00
$\gamma_{lux}=\gamma_{luy}$	1.3	$\gamma_{lux}=\gamma_{luy}$ 1.3
S	0.80	S 0.80
Cx=Cy	0.2184	Cx=Cy 0.2184
Fsx=Fsy	43.10	Fsx=Fsy 43.10

SISMO VINCULADO EN PLANTA BAJA - BLOQUE 4															
Fxt [t]		43.10 CORTE EN NIVEL "X"		Fyt [t]		43.10 CORTE EN NIVEL "Y"									
Ex [m]		-1.06 Lx [m]		13.60 Mtxmax.:		189.00 Mtxmax.:		42.22							
Ey [m]		2.36 Ly [m]		13.50 Mtxmin.:		14.46 Mtxmin.:		-133.62							
DIR.	PLANO "i"	Di [t]	Qp [t]	di [m]	Qmt [t]				Qt [t]	Corte en Muros					
					X		Y			L [cm]	b [cm]	Qm [kg]	τ_m [kg/cm ²]	τ_m [adm]	
					max.	min.	max.	min.							
X	3	81.27	20.45	-4.17	-9.70	-0.74	-2.17	6.86	10.75	945	20	10747	0.63	2.00	
	2	0.44	0.11	3.05	0.04	0.00	0.01	-0.03	0.15	-	-	-	-	-	
	25	0.28	0.07	4.00	0.03	0.00	0.01	-0.02	0.10	-	-	-	-	-	
	22	51.08	12.85	6.33	9.27	0.71	2.07	-6.56	14.92	689	20	14925	1.20	2.00	
	23	38.07	9.58	0.30	0.33	0.03	0.07	-0.24	9.65	425	20	9653	1.26	2.00	
Y	24	0.14	0.03	6.43	0.03	0.00	0.01	-0.02	0.04	-	-	40	-	-	
	25	0.18	0.05	3.15	0.02	0.00	0.00	-0.01	0.05	-	-	51	-	-	
	22	23.85	6.34	-2.85	-1.95	-0.15	-0.44	1.38	5.90	321	20	5900	1.02	2.00	
	23	81.54	21.66	-4.15	-9.70	-0.74	-2.17	6.86	19.49	911	20	19493	1.19	2.00	
	24	0.29	0.08	0.97	0.01	0.00	0.00	-0.01	0.08	-	-	-	-	-	
	21	56.38	14.98	7.20	11.63	0.89	2.60	-8.22	17.57	760	20	17575	1.28	2.00	

6.5.2. ANÁLISIS SÍSMICO EN PLANTA BAJA

SOLICITACIONES SISMICAS EN PLANOS DESVINCULADOS S/1° PISO - BLOQUE 1 (C.C.S. Pcia. De Mza. 1987)							
Co	γ_d	γ_i	$\gamma_{du}(\text{col})$	$\gamma_{du}(\text{mur})$	S	C(col)	C(mur)
0.15	1.4	1.3	1.15	1.3	0.8	0.251	0.284
LOS PLANOS SISMORRESISTENTES QUE CORRESPONDEN AL PRESENTE ANÁLISIS SE ENCUENTRAN ENCUADRADOS EN LA SALVEDAD AL ART. 5.5.5. "ARRIOSTRAMIENTOS DE LAS FUNDACIONES" POR LA EVALUACIÓN EFECTUADA DEL COEFICIENTE γ_i							
DIR.	PLANO	Q [t]	Ts [t]	CORTE EN MUROS			
				Qm [t]	b [cm]	L [cm]	τ_m [kg/cm ²]
X	1	5.07	1.27	---	---	---	---
	2	28.98	7.28	---	---	---	---
	3	31.25	8.87	8.87	20	1825	0.243
Y	4	11.48	3.26	3.26	20	745	0.219
	5	12.57	3.16	---	---	---	---
	6	16.37	4.65	4.65	20	760	0.306
	7	12.82	3.22	---	---	---	---
	8	12.07	3.43	3.43	20	745	0.230

SOLICITACIONES SISMICAS EN PLANOS DESVINCULADOS S/1° PISO - BLOQUE 2 (C.C.S. Pcia. De Mza. 1987)							
Co	γ_d	γ_i	$\gamma_{du}(\text{col})$	$\gamma_{du}(\text{mur})$	S	C(col)	C(mur)
0.15	1.4	1.3	1.15	1.3	0.8	0.251	0.284
LOS PLANOS SISMORRESISTENTES QUE CORRESPONDEN AL PRESENTE ANÁLISIS SE ENCUENTRAN ENCUADRADOS EN LA SALVEDAD AL ART. 5.5.5. "ARRIOSTRAMIENTOS DE LAS FUNDACIONES" POR LA EVALUACIÓN EFECTUADA DEL COEFICIENTE γ_i							
DIR.	PLANO	Q [t]	Ts [t]	CORTE EN MUROS			
				Qm [t]	b [cm]	L [cm]	τ_m [kg/cm ²]
X	1	37.23	9.35	---	---	---	---
	3	33.09	9.40	9.40	20	1780	0.264
Y	9	14.70	4.17	4.17	20	745	0.280
	10	13.64	3.43	---	---	---	---
	11	13.64	3.43	---	---	---	---
	12	13.64	3.43	---	---	---	---
	13	14.70	4.17	4.17	20	745	0.280

CAPITULO 6 – MEMORIA DE CALCULO

SOLICITACIONES SISMICAS EN PLANOS DESVINCULADOS S/1° PISO - BLOQUE 3 (C.C.S. Pcia. De Mza. 1987)							
Co	γ_d	γ_i	$\gamma_{du}(\text{col})$	$\gamma_{du}(\text{mur})$	S	C(col)	C(mur)
0.15	1.4	1.3	1.15	1.3	0.8	0.251	0.284
LOS PLANOS SISMORRESISTENTES QUE CORRESPONDEN AL PRESENTE ANÁLISIS SE ENCUENTRAN ENCUADRADOS EN LA SALVEDAD AL ART. 5.5.5. "ARRIOSTRAMIENTOS DE LAS FUNDACIONES" POR LA EVALUACIÓN EFECTUADA DEL COEFICIENTE γ_i							
DIR.	PLANO	Q [t]	Ts [t]	CORTE EN MUROS			
				Qm [t]	b [cm]	L [cm]	τ_m [kg/cm ²]
X	1	7.12	1.79	---	---	---	---
	2	36.27	9.11	---	---	---	---
	3	40.59	11.52	11.52	20	2725	0.21
Y	14	11.70	3.32	3.32	20	745	0.22
	15	12.42	3.12	---	---	---	---
	16	12.18	3.06	---	---	---	---
	17	12.18	3.06	---	---	---	---
	18	12.18	3.06	---	---	---	---
	19	12.18	3.06	---	---	---	---
	20	11.13	3.16	3.16	20	745	0.21

SOLICITACIONES SISMICAS EN PLANOS DESVINCULADOS S/1° PISO - BLOQUE 4 (C.C.S. Pcia. De Mza. 1987)							
Co	γ_d	γ_i	$\gamma_{du}(\text{col})$	$\gamma_{du}(\text{mur})$	S	C(col)	C(mur)
0.15	1.4	1.3	1.15	1.3	0.8	0.251	0.284
LOS PLANOS SISMORRESISTENTES QUE CORRESPONDEN AL PRESENTE ANÁLISIS SE ENCUENTRAN ENCUADRADOS EN LA SALVEDAD AL ART. 5.5.5. "ARRIOSTRAMIENTOS DE LAS FUNDACIONES" POR LA EVALUACIÓN EFECTUADA DEL COEFICIENTE γ_i							
DIR.	PLANO	Q [t]	Ts [t]	CORTE EN MUROS			
				Qm [t]	b [cm]	L [cm]	τ_m [kg/cm ²]
X	1	0.77	0.19	---	---	---	---
	3	13.98	3.97	3.97	20	945	0.21
	2	6.20	1.56	---	---	---	---
	25	4.38	1.10	---	---	---	---
	22	15.51	4.40	4.40	20	688.56	0.32
	23	6.13	1.74	1.74	20	425.115	0.20
	24	2.87	0.72	---	---	---	---
	26	0.35	0.09	---	---	---	---
Y	25	2.94	0.74	---	---	---	---
	22	8.82	2.50	2.50	20	321.48	0.39
	23	14.11	4.00	4.00	20	910.53	0.22
	24	6.60	1.66	---	---	---	---
	21	16.91	4.80	4.80	20	760	0.32
	26	0.81	0.20	---	---	---	---

6.5.3. PLANILLA DE BASES

BASES DE H°A° - BLOQUE 1											
DESIGNAC.	ax [cm]	ay [cm]	h [cm]	M [tcm]	N [t]	Q [t]	σ_1 [kg/cm ²]	σ_2 [kg/cm ²]	σ_3 [kg/cm ²]	σ_4 [kg/cm ²]	A° INF.
CIMENTO CORRIDO	ANCHO: 50		60	---	---	---	---	---	---	---	HORMIG.
											CICLÓPEO
B1	110	110	100	287	-3.2	0.69	1.00	0.81	1.59	0.53 FICT.	#Ø12 C/15
B2	240	170	120	2120	-20.5	8.19	1.67	0.79	2.37	1.19 FICT.	#Ø12 C/15
B3	140	140	110	263	-23.65	1.21	1.00	1.67	2.31	0.63 FICT.	#Ø12 C/15

BASES DE H°A° - BLOQUE 2											
DESIGNAC.	ax [cm]	ay [cm]	h [cm]	M [tcm]	N [t]	Q [t]	σ_1 [kg/cm ²]	σ_2 [kg/cm ²]	σ_3 [kg/cm ²]	σ_4 [kg/cm ²]	A° INF.
CIMENTO CORRIDO	ANCHO: 50		60	---	---	---	---	---	---	---	HORMIG.
											CICLÓPEO
B0	80	80	70	---	---	---	---	---	---	---	#Ø8 C/15
B2	240	170	120	2120	-20.5	8.19	1.67	0.79	2.37	1.19 FICT.	#Ø12 C/15
B3	140	140	110	263	-23.65	1.21	1.00	1.67	2.31	0.63 FICT.	#Ø12 C/15

BASES DE H°A° - BLOQUE 3											
DESIGNAC.	ax [cm]	ay [cm]	h [cm]	M [tcm]	N [t]	Q [t]	σ_1 [kg/cm ²]	σ_2 [kg/cm ²]	σ_3 [kg/cm ²]	σ_4 [kg/cm ²]	A° INF.
CIMENTO CORRIDO	ANCHO: 50		60	---	---	---	---	---	---	---	HORMIG.
											CICLÓPEO
B0	80	80	70	---	---	---	---	---	---	---	#Ø8 C/15
B1	110	110	100	287	-3.2	0.69	1.00	0.81	1.59	0.53 FICT.	#Ø12 C/15
B2	240	170	120	2120	-20.5	8.19	1.67	0.79	2.37	1.19 FICT.	#Ø12 C/15
B3	140	140	110	263	-23.65	1.21	1.00	1.67	2.31	0.63 FICT.	#Ø12 C/15

BASES DE H°A° - BLOQUE 4											
DESIGNAC.	ax [cm]	ay [cm]	h [cm]	M [tcm]	N [t]	Q [t]	σ_1 [kg/cm ²]	σ_2 [kg/cm ²]	σ_3 [kg/cm ²]	σ_4 [kg/cm ²]	A° INF.
CIMENTO CORRIDO	ANCHO: 50		60	---	---	---	---	---	---	---	HORMIG.
											CICLÓPEO
B0	80	80	70	---	---	---	---	---	---	---	#Ø8 C/15

BASES DE H°A° - ESCALERA EXTERIOR											
DESIGNAC.	ax [cm]	ay [cm]	h [cm]	M [tcm]	N [t]	Q [t]	σ_1 [kg/cm ²]	σ_2 [kg/cm ²]	σ_3 [kg/cm ²]	σ_4 [kg/cm ²]	A° INF.
B4	550	200	100	957	13.6	2.9	1.33	1.04	0.43	0.24	#Ø12 C/15

CAPITULO 6 – MEMORIA DE CALCULO

6.5.4. PLANILLA DE COLUMNAS Y TABIQUES

COLUMNAS DE Hº Aº - BLOQUE 1													
DESIG.	DIMENSIONES		SOLICITACIONES			Aº LONGITUDINAL			ESTRIBOS				
	ax [cm]	ay [cm]	Mmax [tcm]	Nmax [t]	Q [t]	A=A' [cm²]	INT. Aº		Vcb [t]	Vs [t]	Ø [mm]	Ast [cm²]	SEP. [cm]
							A	A'					
Cv	18	18	---	---	---	---	2 Ø 8	2 Ø 8	---	---	6	0.28	20
C1	30		Y=81 X=287	-3.2	0.49 0.69	16.08	8 Ø 16		5.27	Ast min	6	0.28	20
C2	80	20	2120	-20.5	8.19	9.42 2.2	3 Ø 20	3 Ø 20 2 Ø 12	11.88	Ast min	6	0.28	20
C3	25	46	1325	-14.37	4.00	15.45	3Ø12e + 6Ø16	3Ø12e + 6Ø16	8.47	Ast min	6	0.28	20
C4	25	46	671.5	-17.29	2.03	11.43	3Ø12 + 4Ø16	3Ø12 + 4Ø16	8.47	Ast min	6	0.28	20
C5	30		Y=158 X=263	-23.65	1.21 0.96	43.96	14 Ø 20		5.27	Ast min	8	0.5	20
C101	30		Y=76.7 X=182	-1.63	0.89 0.25	16.08	8 Ø 16		5.27	Ast min	6	0.28	20
C102	80	20	203	-7.18	0.46	9.42 2.2	3 Ø 20	3 Ø 20 2 Ø 12	11.88	Ast min	6	0.28	20
C103	25	46	1075	-3.54	2.11	15.45	3Ø12e + 6Ø16	3Ø12e + 6Ø16	8.47	Ast min	6	0.28	20
C104	25	46	835	-4.75	3.25	11.43	3Ø12 + 4Ø16	3Ø12 + 4Ø16	8.47	Ast min	6	0.28	20
C105	30		Y=561 X=638	-5.32	1.75 1.87	43.96	14 Ø 20		5.27	Ast min	8	0.5	20

COLUMNAS DE Hº Aº - BLOQUE 2													
DESIG.	DIMENSIONES		SOLICITACIONES			Aº LONGITUDINAL			ESTRIBOS				
	ax [cm]	ay [cm]	Mmax [tcm]	Nmax [t]	Q [t]	A=A' [cm²]	INT. Aº		Vcb [t]	Vs [t]	Ø [mm]	Ast [cm²]	SEP. [cm]
							A	A'					
Cv	18	18	---	---	---	---	2 Ø 8	2 Ø 8	---	---	6	0.28	20
C1	30		Y=81 X=287	-3.2	0.49 0.69	16.08	8 Ø 16		5.27	Ast min	6	0.28	20
C2	80	18	2120	-20.5	8.19	9.42 2.2	3 Ø 20	3 Ø 20 2 Ø 12	10.69	0.23	6	0.28	20
C3	25	46	1325	-14.37	4.00	15.45	3Ø12e + 6Ø16	3Ø12e + 6Ø16	8.47	Ast min	6	0.28	20
C5	30		Y=158 X=263	-23.65	1.21 0.96	43.96	14 Ø 20		5.27	Ast min	8	0.5	20
C101	30		Y=76.7 X=182	-1.63	0.89 0.25	16.08	8 Ø 16		5.27	Ast min	6	0.28	20
C102	80	18	203	-7.18	0.46	9.42 2.2	3 Ø 20	3 Ø 20 2 Ø 12	10.69	Ast min	6	0.28	20
C103	25	46	1075	-3.54	2.11	15.45	3Ø12e + 6Ø16	3Ø12e + 6Ø16	8.47	Ast min	6	0.28	20
C105	30		Y=561 X=638	-5.32	1.75 1.87	43.96	14 Ø 20		5.27	Ast min	8	0.5	20

COLUMNAS DE Hº Aº - BLOQUE 3													
DESIG.	DIMENSIONES		SOLICITACIONES			Aº LONGITUDINAL			ESTRIBOS				
	ax [cm]	ay [cm]	Mmax [tcm]	Nmax [t]	Q [t]	A=A' [cm²]	INT. Aº		Vcb [t]	Vs [t]	Ø [mm]	Ast [cm²]	SEP. [cm]
							A	A'					
Cv	18	18	---	---	---	---	2 Ø 8	2 Ø 8	---	---	6	0.28	20
C1	30		Y=81 X=287	-3.2	0.49 0.69	16.08	8 Ø 16		5.27	Ast min	6	0.28	20
C2	80	20	2120	-20.5	8.19	9.42 2.2	3 Ø 20	3 Ø 20 2 Ø 12	11.88	Ast min	6	0.28	20
C3	25	46	1325	-14.37	4.00	15.45	3Ø12e + 6Ø16	3Ø12e + 6Ø16	8.47	Ast min	6	0.28	20
C4	25	46	671.5	-17.29	2.03	11.43	3Ø12 + 4Ø16	3Ø12 + 4Ø16	8.47	Ast min	6	0.28	20
C5	30		Y=158 X=263	-23.65	1.21 0.96	43.96	14 Ø 20		5.27	Ast min	8	0.5	20
C101	30		Y=76.7 X=182	-1.63	0.89 0.25	16.08	8 Ø 16		5.27	Ast min	6	0.28	20
C102	80	20	203	-7.18	0.46	9.42 2.2	3 Ø 20	3 Ø 20 2 Ø 12	11.88	Ast min	6	0.28	20
C103	25	46	1075	-3.54	2.11	15.45	3Ø12e + 6Ø16	3Ø12e + 6Ø16	8.47	Ast min	6	0.28	20
C104	25	46	835	-4.75	3.25	11.43	3Ø12 + 4Ø16	3Ø12 + 4Ø16	8.47	Ast min	6	0.28	20
C105	30		Y=561 X=638	-5.32	1.75 1.87	43.96	14 Ø 20		5.27	Ast min	8	0.5	20

CAPITULO 6 – MEMORIA DE CALCULO

COLUMNAS DE Hº Aº - BLOQUE 4													
DESIG.	DIMENSIONES		SOLICITACIONES			Aº LONGITUDINAL			ESTRIBOS				
	ax [cm]	ay [cm]	Mmax [tcm]	Nmax [t]	Q [t]	A=A' [cm2]	INT. Aº		Vcb [t]	Vs [t]	Ø [mm]	Ast [cm²]	SEP. [cm]
							A	A'					
Cv	18	18	---	---	---	---	2 Ø 8	2 Ø 8	---	---	6	0.28	20
C6	18	40	Y=94	-25.59	0.24	19.07	2Ø12e + 1Ø10e	2Ø12e + 1Ø10e	1.90	Ast mín	8	0.5	20
							+ 8Ø16	+ 8Ø16					
C7	30		169	-5.25	0.51	3.16	INTERMEDIA 2Ø12e		5.27	Ast mín	6	0.28	20
C106	18	40	Y=632	-2.62	2.22	19.07	2Ø12e + 1Ø10e	2Ø12e + 1Ø10e	1.90	1.06	8	0.5	20
							+ 8Ø16	+ 8Ø16					
C107	30		149	-0.98	0.54	3.16	INTERMEDIA 2Ø12e		5.27	Ast mín	6	0.28	20
C108	18	40	---	-30	---	---	3 Ø 12	3 Ø 12	---	---	6	0.28	20
							INTERMEDIA 2 Ø 10						

TABIQUES DE Hº Aº - ESCALERA EXTERIOR													
DESIG.	DIMENSIONES		SOLICITACIONES			Aº LONGITUDINAL			ESTRIBOS				
	ax [cm]	ay [cm]	Mmax [tcm]	Nmax [t]	Q [t]	A=A' [cm2]	INT. Aº		Vcb [t]	Vs [t]	Ø [mm]	Ast [cm²]	SEP. [cm]
							A	A'					
T1	100	18	211	2.55	0.64	11.31	1 Ø 12 c/10cm	1 Ø 12 c/10cm	58.51	Ast mín	6	0.28	20

6.5.5. PLANILLA DE LOSAS CERÁMICAS

LOSAS CERÁMICAS (TIPO PREAR 90) - BLOQUE 1							
DESIG.	q [kg/m2]	L [m]	M [kgm]	SERIE VIGUETA	ESP. DE C. DE C. [cm]	ALT. BLOKE [cm]	Madm. [kgm]
L1	760	4.50	1924	S5º de 4.60 m	5	16.5	2046

LOSAS CERÁMICAS (TIPO PREAR 90) - BLOQUE 2							
DESIG.	q [kg/m2]	L [m]	M [kgm]	SERIE VIGUETA	ESP. DE C. DE C. [cm]	ALT. BLOKE [cm]	Madm. [kgm]
L1	760	4.50	1924	S5º de 4.60 m	5	16.5	2046

LOSAS CERÁMICAS (TIPO PREAR 90) - BLOQUE 3							
DESIG.	q [kg/m2]	L [m]	M [kgm]	SERIE VIGUETA	ESP. DE C. DE C. [cm]	ALT. BLOKE [cm]	Madm. [kgm]
L1	760	4.50	1924	S5º de 4.60 m	5	16.5	2046

LOSAS CERÁMICAS (TIPO PREAR 90) - BLOQUE 4							
DESIG.	q [kg/m2]	L [m]	M [kgm]	SERIE VIGUETA	ESP. DE C. DE C. [cm]	ALT. BLOKE [cm]	Madm. [kgm]
L1	760	4.50	1924	S5º de 4.60 m	5	16.5	2046
L2	760	5.75	3141	DOBLE S6º de 5.90 m (máx)	5	16.5	3743
L3	760	5.95	3363	DOBLE S6º de 6.10 m (máx)	5	16.5	3743
L4	760	6.25	3711	DOBLE S6º de 6.40 m (máx)	5	16.5	3743

CAPITULO 6 – MEMORIA DE CALCULO

6.5.6. PLANILLA DE VIGAS Y LOSAS DE HORMIGÓN ARMADO

DESIGN.	LUZ [m]	SOLICITACIONES		M [cm]			DIMENSIONES [m]			SECCION DE Aº [cm²]			INTEGRACION DE ARMADURA			Vua [t]	Vca [t]	Vsa [t]	ESTRIBO			BARRAS A 45º					
		Diagrama	q [kg/m]	A	Tramo		dmín cm	b cm	h mín cm	A	B	Tramo	Sup. Inf.	B	Sup. Inf.				Ø [mm]	Ast [cm²]	Sep. [cm]	INT.	XA [cm]	B	INT.	XB [cm]	
					Neg. Pos.	Neg. Pos.																					Sup. Inf.
Vf1	18	27	30	2 Ø 8	2 Ø 8	6	20
Vf2	18	27	30	2 Ø 12	2 Ø 12	6	20
Vd-VE	18	17	20	2 Ø 8	2 Ø 8	6	20
VE2	18	27	30	2 Ø 8	2 Ø 8	6	20
V1a	4.5		550	121	...	123	18	37	40	2.2	...	2.2	2 Ø 12	2 Ø 12	2 Ø 12	2 Ø 12	1.62	4.96	Ast mín	6	0.28	20
V1b	4.5		550	123	...	123	18	37	40	2.2	...	2.2	2 Ø 12	2 Ø 12	2 Ø 12	2 Ø 12	1.62	4.96	Ast mín	6	0.28	20
V1c	4.5		550	123	...	123	18	37	40	2.2	...	2.2	2 Ø 12	2 Ø 12	2 Ø 12	2 Ø 12	1.62	4.96	Ast mín	6	0.28	20
V1d	4.5		550	123	...	121	18	37	40	2.2	...	2.2	2 Ø 12	2 Ø 12	2 Ø 12	2 Ø 12	1.62	4.96	Ast mín	6	0.28	20
V2a	4.5		1685	253	...	253	18	57	60	3.39	...	3.39	3 Ø 12	3 Ø 12	3 Ø 12	3 Ø 12	3.25	7.65	Ast mín	6	0.28	20
V2b	4.5		300	253	18	57	60	3.39	...	3.39	3 Ø 12	3 Ø 12	2 Ø 12	2 Ø 12	1.25	7.65	Ast mín	6	0.28	20
V2c	4.5		300	77	18	57	60	2.2	...	2.2	2 Ø 12	2 Ø 12	2 Ø 12	2 Ø 12	0.86	7.65	Ast mín	6	0.28	20
V2d	4.5		300	42	18	57	60	2.2	...	2.2	2 Ø 12	2 Ø 12	2 Ø 12	2 Ø 12	0.5	7.65	Ast mín	6	0.28	20
V3	3		1970	222	25	47	50	2.2	2 Ø 12	2 Ø 12	2 Ø 12	2 Ø 12	2.95	8.76	Ast mín	6	0.28	20
V4a	4.5		4000	1432	...	1868	25	47	50	10.3	...	19.72	2 Ø 16+2 Ø 20	2 Ø 16	2 Ø 16+5 Ø 20	14.72	8.76	10.87	8	0.5	18.16	2 Ø 20	2 Ø 20	2 Ø 20	150	100	
V4b	3		4000	805	...	86	25	47	50	19.72	...	7.16	2 Ø 16+5 Ø 20	2 Ø 16+1 Ø 20	2 Ø 16+1 Ø 20	8.85	8.76	3.04	6	0.28	20	2 Ø 20	2 Ø 20	100	100	...	
V5a	3		1990	398	25	47	50	3.39	2 Ø 12	2 Ø 12	4 Ø 12	1.66	8.76	Ast mín	6	0.28	20	2 Ø 20	50	
V5b	2		1990	398	25	47	50	3.39	4 Ø 12	2 Ø 12	2 Ø 12	3.98	8.76	Ast mín	6	0.28	20	2 Ø 20	2 Ø 20	50	

CAPITULO 6 – MEMORIA DE CALCULO

DESIGN	LUZ [m]	SOLICITACIONES	M [tcm]		DIMENSIONES [m]			SECCION DE A° [cm2]			INTEGRACION DE ARMADURA			Vua [t]	Vcb [t]	Vsa [t]	ESTRIBO		BARRAS A 45°			
		Diagrama	A	B	b	dmin	hmin	A	Tr.	B	A	Tramo	B				Ø	Ast [cm²]	Sep. [cm]	A	B	
		q [kg/m]	Neg. Pos.	Neg. Pos.	cm	cm	cm	Sup. Inf.	Sup. Inf.	Sup. Inf.	Sup. Inf.	Sup. Inf.	Sup. Inf.				[mm]			INT.	XA [cm]	XB [cm]
V101a	4.5		66	34	66	18	37	40	1.57	1.57	1.57	2 Ø 10	2 Ø 10	0.89	4.96	Ast mín	6	0.28	20	---	---	---
V101b	4.5		66	33	68	18	37	40	1.57	1.57	1.57	2 Ø 10	2 Ø 10	0.89	4.96	Ast mín	6	0.28	20	---	---	---
V101c	4.5		68	33	66	18	37	40	1.57	1.57	1.57	2 Ø 10	2 Ø 10	0.89	4.96	Ast mín	6	0.28	20	---	---	---
V101d	4.5		66	34	66	18	37	40	1.57	1.57	1.57	2 Ø 10	2 Ø 10	0.89	4.96	Ast mín	6	0.28	20	---	---	---
VP	4.5		---	76	---	18	57	60	1.57	1.57	1.57	3 Ø 10	3 Ø 10	0.88	7.65	Ast mín	6	0.28	20	---	---	---
LM1	1.95		---	---	---	100	15	18	---	---	---	1 Ø 16c/8.5cm	1 Ø 16c/8.5cm	6.55	---	---	2.7	1 Ø 6c/10cm	REPARTICION	---	---	---

VIGAS Y LOSAS DE H°A° - BLOQUE 2																						
DESIGN	LUZ [m]	SOLICITACIONES	M [tcm]		DIMENSIONES [m]			SECCION DE A° [cm2]			INTEGRACION DE ARMADURA			Vua [t]	Vcb [t]	Vsa [t]	ESTRIBO		BARRAS A 45°			
			A	B	b	dmin	hmin	A	Tr.	B	A	Tramo	B				Ø	Ast [cm²]	Sep. [cm]	A	B	
		q [kg/m]	Neg. Pos.	Neg. Pos.	cm	cm	cm	Sup. Inf.	Sup. Inf.	Sup. Inf.	Sup. Inf.	Sup. Inf.	Sup. Inf.				[mm]			INT.	XA [cm]	XB [cm]
Vf1	---	---	---	---	18	27	30	---	---	---	2 Ø 8	2 Ø 8	2 Ø 8	---	---	---	---	6	20	---	---	---
Vf2	---	---	---	---	18	27	30	---	---	---	2 Ø 12	2 Ø 12	2 Ø 12	---	---	---	---	6	20	---	---	---
Vd-VE	---	---	---	---	18	17	20	---	---	---	2 Ø 8	2 Ø 8	2 Ø 8	---	---	---	---	6	20	---	---	---
VE2	---		---	---	18	27	30	---	---	---	2 Ø 8	2 Ø 8	2 Ø 8	---	---	---	---	6	20	---	---	---
V5a	3		---	398	25	47	50	---	3.39	3.39	2.2	2 Ø 12	2 Ø 12	1.66	8.76	Ast mín	6	0.28	20	---	---	2 Ø 20
V5b	2		398	---	25	47	50	3.39	---	---	2.2	2 Ø 12	2 Ø 12	3.98	8.76	Ast mín	6	0.28	20	2 Ø 20	50	---
V6a	4.5		125	---	18	37	40	2.2	2.2	2.2	2 Ø 12	2 Ø 12	2 Ø 12	1.58	4.96	Ast mín	6	0.28	20	---	---	---
V6b	4.5		125	123	18	37	40	2.2	2.2	2.2	2 Ø 12	2 Ø 12	2 Ø 12	1.62	4.96	Ast mín	6	0.28	20	---	---	---
V6c	4.5		123	---	18	37	40	2.2	2.2	2.2	2 Ø 12	2 Ø 12	2 Ø 12	1.6	4.96	Ast mín	6	0.28	20	---	---	---
V6d	4.5		125	---	18	37	40	2.2	2.2	2.2	2 Ø 12	2 Ø 12	2 Ø 12	1.65	4.96	Ast mín	6	0.28	20	---	---	---

CAPITULO 6 – MEMORIA DE CALCULO

V1a	4.5		550	121	---	123	18	37	40	2.2	---	2.2	---	2.2	2 Ø 12	2 Ø 12	2 Ø 12	1.62	4.96	Ast mín	6	0.28	20	---	---	---	---
V1b-V1c- V1d-V1e	4.5		550	123	---	123	18	37	40	2.2	---	2.2	---	2.2	2 Ø 12	2 Ø 12	2 Ø 12	1.62	4.96	Ast mín	6	0.28	20	---	---	---	---
V1f	4.5		550	123	---	121	18	37	40	2.2	---	2.2	---	2.2	2 Ø 12	2 Ø 12	2 Ø 12	1.62	4.96	Ast mín	6	0.28	20	---	---	---	---
V9a	4.5		300	65	---	65	18	57	60	2.2	---	2.2	---	2.2	2 Ø 12	2 Ø 12	2 Ø 12	1.62	7.65	Ast mín	6	0.28	20	---	---	---	---
V9b	4.5		300	65	---	44	18	57	60	2.2	---	2.2	---	2.2	2 Ø 12	2 Ø 12	2 Ø 12	1.62	7.65	Ast mín	6	0.28	20	---	---	---	---
V9c	4.5		300	44	---	66	18	57	60	2.2	---	2.2	---	2.2	2 Ø 12	2 Ø 12	2 Ø 12	1.62	7.65	Ast mín	6	0.28	20	---	---	---	---
V9d	4.5		300	66	---	48	18	57	60	2.2	---	2.2	---	2.2	2 Ø 12	2 Ø 12	2 Ø 12	1.62	7.65	Ast mín	6	0.28	20	---	---	---	---
V9e	4.5		300	---	---	252	18	57	60	2.2	---	2.2	---	2.2	2 Ø 12	2 Ø 12	2 Ø 12	1.62	7.65	Ast mín	6	0.28	20	---	---	---	---
V9f	4.5		1695	252	---	252	18	57	60	3.39	---	3.39	---	3.39	3 Ø 12	3 Ø 12	3 Ø 12	3.25	7.65	Ast mín	6	0.28	20	---	---	---	---
V3	3		1970	---	---	---	25	47	50	---	---	2.2	---	2.2	2 Ø 12	2 Ø 12	2 Ø 12	2.95	8.76	Ast mín	6	0.28	20	---	---	---	---
V4a	4.5		4000	1432	---	1868	25	47	50	10.3	---	19.72	---	19.72	2 Ø 16+2 Ø 20	2 Ø 16	2 Ø 16+5 Ø 20	14.72	8.76	10.87	8	0.5	18.16	2 Ø 20	2 Ø 20	2 Ø 20	120
V4b	3		4000	---	---	1110	25	47	50	---	---	13.44	---	13.44	2 Ø 16+1 Ø 20	2 Ø 16+3 Ø 20	2 Ø 16+1 Ø 20	15.93	8.76	3.04	6	0.28	20	2 Ø 20	2 Ø 20	100	100
V5a	3		1990	---	---	388	25	47	50	---	---	3.39	---	3.39	2 Ø 12	2 Ø 12	2 Ø 12	1.66	8.76	Ast mín	6	0.28	20	---	---	---	50
V5b	2		1990	398	---	---	25	47	50	3.39	---	---	---	---	4 Ø 12	2 Ø 12	2 Ø 12	3.98	8.76	Ast mín	6	0.28	20	2 Ø 20	50	---	---
V101a	4.5		200	66	---	66	18	37	40	1.57	---	1.57	---	1.57	2 Ø 10	2 Ø 10	2 Ø 10	0.89	4.96	Ast mín	6	0.28	20	---	---	---	---
V101b-V101c- V101d-V101e	4.5		200	66	---	66	18	37	40	1.57	---	1.57	---	1.57	2 Ø 10	2 Ø 10	2 Ø 10	0.89	4.96	Ast mín	6	0.28	20	---	---	---	---
V101f	4.5		200	66	---	66	18	37	40	1.57	---	1.57	---	1.57	2 Ø 10	2 Ø 10	2 Ø 10	0.89	4.96	Ast mín	6	0.28	20	---	---	---	---
VP	4.5		300	---	---	---	18	57	60	---	---	---	---	---	2 Ø 10	2 Ø 10	2 Ø 10	0.88	7.65	Ast mín	6	0.28	20	---	---	---	---
LM1	1.95		3205	---	---	---	100	15	18	---	---	---	---	---	1 Ø 16c/8.5cm	1 Ø 16c/8.5cm	1 Ø 16c/8.5cm	6.85	---	---	2.7	1 Ø 6 c/10cm	---	---	---	---	

CAPITULO 6 – MEMORIA DE CALCULO

VIGAS Y LOSAS DE H^aA - BLOQUE 4

DESIGN.	LUZ [m]	SOLICITACIONES		M [tcm]			DIMENSIONES [m]			SECCION DE A ^o [cm ²]						INTEGRACION DE ARMADURA						Vca [t]			ESTRIBO			BARRAS A 45°						
		Diagrama	q [kg/m]	A		b	dmín	h	Tramo	Neg. Pos.	B	Sup. Inf.	A	Tramo	Sup. Inf.	B	Sup. Inf.	Vua [t]	Vcb [t]	Vsa [t]	Ø [mm]	Ast [cm ²]	Sep. [cm]	INT.	XA [cm]	B	XB [cm]	INT.	A	XA [cm]	B			
				Sup. Inf.	Sup. Inf.																											Sup. Inf.	Sup. Inf.	Sup. Inf.
Vf1	18	27	30	2 Ø 8	2 Ø 8	2 Ø 8	2 Ø 8	2 Ø 8	2 Ø 8	6	20		
Vf2	18	27	30	2 Ø 12	2 Ø 12	2 Ø 12	2 Ø 12	2 Ø 12	2 Ø 12	6	20		
Vd+VE	18	17	20	2 Ø 8	2 Ø 8	2 Ø 8	2 Ø 8	2 Ø 8	2 Ø 8	6	20	
VE2	18	27	30	2 Ø 8	2 Ø 8	2 Ø 8	2 Ø 8	2 Ø 8	2 Ø 8	0.64	6	20	
V10	3	...	200	...	18	37	40	23	2 Ø 10	2 Ø 10	2 Ø 10	2 Ø 10	2 Ø 10	2 Ø 10	0.3	4.96	0.28	20	
V11	3	...	1700	...	18	57	60	191	3 Ø 10	3 Ø 10	3 Ø 10	3 Ø 10	3 Ø 10	3 Ø 10	2.55	7.65	0.28	20	1 Ø 10	50	1 Ø 10	50	1 Ø 10	50	1 Ø 10	50	1 Ø 10	50	50	
V12	3.75	...	730	...	18	28	30	129	3 Ø 10	3 Ø 10	3 Ø 10	3 Ø 10	3 Ø 10	3 Ø 10	1.37	3.76	0.28	20	1 Ø 10	65	1 Ø 10	65	1 Ø 10	65	1 Ø 10	65	1 Ø 10	65	65	
V13	7.7	...	4225	...	25	67	70	3131	4 Ø 20	2 Ø 20	4 Ø 20	4 Ø 20	4 Ø 20	16.26	12.48	9.20	2.16	1 Ø 6 c/10cm	
LM3	3	...	3030	...	100	12	15	341	1 Ø 12c/8cm	1 Ø 12c/8cm	1 Ø 12c/8cm	1 Ø 12c/8cm	1 Ø 12c/8cm	4.5
LM4	2.55	...	1720	...	100	12	15	103	1 Ø 8 c/10cm	1 Ø 8 c/10cm	1 Ø 8 c/10cm	1 Ø 8 c/10cm	1 Ø 8 c/10cm	2.2

VIGAS Y LOSAS DE H^aA - ESCALERA DE EXTERIOR

DESIGN.	LUZ [m]	SOLICITACIONES		M [tcm]			DIMENSIONES [m]			SECCION DE A ^o [cm ²]						INTEGRACION DE ARMADURA						Vca [t]			ESTRIBO			BARRAS A 45°							
		Diagrama	q [kg/m]	A		b	dmín	h	Tramo	Neg. Pos.	B	Sup. Inf.	A	Tramo	Sup. Inf.	B	Sup. Inf.	Vua [t]	Vcb [t]	Vsa [t]	Ø [mm]	Ast [cm ²]	Sep. [cm]	INT.	XA [cm]	B	XB [cm]	INT.	A	XA [cm]	B				
				Sup. Inf.	Sup. Inf.																											Sup. Inf.	Sup. Inf.	Sup. Inf.	Sup. Inf.
Vf2	18	27	30	2 Ø 12	2 Ø 12	2 Ø 12	2 Ø 12	2 Ø 12	6	20	
Ve	415	...	48	13.5	16.5	47	4 Ø 8	4 Ø 8	4 Ø 8	4 Ø 8	4 Ø 8	0.82	4.83	0.28	20	
Le	1270	...	100	15	18	163	1 Ø 8c/10cm	1 Ø 8c/10cm	1 Ø 8c/10cm	1 Ø 8c/10cm	1 Ø 8c/10cm	2.03	
Vv	18	15	18	2 Ø 8	2 Ø 8	2 Ø 8	2 Ø 8	2 Ø 8

6.5.7. PLANILLAS DE CORREAS DE TECHO

CORREAS DE TECHO - BLOQUE 1														
DESIG.	SEP. [m]	qc [kg/cm]	TIPO DE ESTR.	α [°]	Mx [kgcm]	My [kgcm]	SECCIÓN ADOPTADA	Wx [cm ³]	Wy [cm ³]	σ_{MAX} [kg/cm ²]	Jx [cm ⁴]	Jy [cm ⁴]	f [cm]	L/250 [cm]
C1	0.80	1.12	CONTINUA L1=L2=L3=L4=L5=4.50	7	24054	3034	C 140 – 60 – 2.00	25.2	7.51	1359	176.4	29.35	0.78	1.80

CORREAS DE TECHO - BLOQUE 2														
DESIG.	SEP. [m]	qc [kg/cm]	TIPO DE ESTR.	α [°]	Mx [kgcm]	My [kgcm]	SECCIÓN ADOPTADA	Wx [cm ³]	Wy [cm ³]	σ_{MAX} [kg/cm ²]	Jx [cm ⁴]	Jy [cm ⁴]	f [cm]	L/250 [cm]
C1	0.80	1.12	CONTINUA L1=L2=L3=L4=L5=4.50	7	24054	3034	C 140 – 60 – 2.00	25.2	7.51	1359	176.4	29.35	0.78	1.80

CORREAS DE TECHO - BLOQUE 3														
DESIG.	SEP. [m]	qc [kg/cm]	TIPO DE ESTR.	α [°]	Mx [kgcm]	My [kgcm]	SECCIÓN ADOPTADA	Wx [cm ³]	Wy [cm ³]	σ_{MAX} [kg/cm ²]	Jx [cm ⁴]	Jy [cm ⁴]	f [cm]	L/250 [cm]
C2	0.80	1.12	CONTINUA L1=L2=L3=L4=L5=L6=4.50	7	23749	2995	C 140 – 60 – 2.00	25.2	7.51	1341	176.4	29.35	0.81	1.80

CORREAS DE TECHO - BLOQUE 4														
DESIG.	SEP. [m]	qc [kg/cm]	TIPO DE ESTR.	α [°]	Mx [kgcm]	My [kgcm]	SECCIÓN ADOPTADA	Wx [cm ³]	Wy [cm ³]	σ_{MAX} [kg/cm ²]	Jx [cm ⁴]	Jy [cm ⁴]	f [cm]	L/250 [cm]
C2	0.80	1.12	CONTINUA L-VARIABLE	7	23749	2995	C 140 – 60 – 2.00	25.2	7.51	1341	176.4	29.35	0.81	1.80

6.5.8. PLANILLAS DE VIGAS RETICULADAS

CERCHAS Y PERFILES DE ACERO - BLOQUE 1																	
DESIGN.	BARRA	L [cm]	N [kg]	M [tcm]	SECCION ADOPTADA	Ag [cm2]	Jx [cm4]	Y [cm]	σmáx. [kg/cm2]	r [cm]		Lkx [cm]		λx		σ min. [kg/cm2]	
										IND.	CONJ.	IND.	CONJ.	IND.	CONJ.		
VR1	C. SUP.	48	-9330	---	2 ANG. 2" x 1/4" (51mm x 6.4mm)	12.34	---	---	---	---	---	---	48	---	38	1.3	-983
	C. INF.	48	9330	---	2 ANG. 2" x 1/4" (51mm x 6.4mm)	12.34	---	---	756	---	---	---	---	---	---	---	---
	ZIG-ZAG LATERAL	51	-1348	---	Ø 20	3.14	---	---	---	0.5	---	---	48	---	96	1.82	-781
	ZIG-ZAG SUP. E INF.	23	-618	---	Ø 12	1.13	---	---	---	0.3	---	---	21.3	---	71	1.42	-777
VR2	C. SUP.	32	-2570	---	2 ANG. 1" x 1/8" (25mm x 3.2mm)	3.02	---	---	---	---	---	---	---	---	43	1.16	-987
	C. INF.	32	2570	---	2 ANG. 1" x 1/8" (25mm x 3.2mm)	3.02	---	---	851	---	---	---	---	---	---	---	---
	ZIG-ZAG LATERAL	34	-578	---	Ø 12	1.13	---	---	---	0.3	---	---	31.9	---	106	2.02	-1033.2
	ZIG-ZAG SUP. E INF.	23	-277	---	Ø 8	0.5	---	---	---	0.2	---	---	21.3	---	107	2.05	-1135.7
VM1	MÉNSULA	200	170	137	C 220-80-2.5 DOBLE	20.6	1531	11	993	---	---	---	---	---	---	---	---

CERCHAS Y PERFILES DE ACERO - BLOQUE 2																		
DESIGN.	BARRA	L [cm]	N [kg]	M [tcm]	SECCION ADOPTADA	Ag [cm2]	Jx [cm4]	Y [cm]	σmáx. [kg/cm2]	r [cm]		Lkx [cm]		λx		σ min. [kg/cm2]		
										IND.	CONJ.	IND.	CONJ.	IND.	CONJ.			
VR3	C. SUP.	106	-12605	---	2 ANG. 2 1/2" x 3/16" (64mm x 4.8mm)	12	---	---	---	---	1.2	1.95	53*	106	44	54	1.24	-1303
	C. INF.	212	12138	---	2 ANG. 2 1/2" x 3/16" (64mm x 4.8mm)	12	---	---	1012	---	---	---	---	---	---	---	---	---
	MONTS.	66	-666	---	□ 50-50-2.00	3.03	---	---	---	---	---	1.95	---	66	---	34	1.1	-242
	DIAGS.	124	4752	---	□ 50-50-2.00	3.03	---	---	1568	---	---	---	---	---	---	---	---	---
VM1	MÉNSULA	200	170	137	C 220-80-2.5 DOBLE	20.6	1531	11	993	---	---	---	---	---	---	---	---	
VM2	MÉNSULA	300	252	308	C 220-80-4.8 DOBLE	38.2	2732	11	1247	---	---	---	---	---	---	---	---	

* Ajustar a la mitad de la longitud

CERCHAS Y PERFILES DE ACERO - BLOQUE 3																	
DESIGN	BARRA	L [cm]	N [kg]	M [tcm]	SECCION ADOPTADA	Ag [cm ²]	Jx [cm ⁴]	Y [cm]	σ máx. [kg/cm ²]	r [cm]		Lkx [cm]		λx		ω máx	σ mín. [kg/cm ²]
										IND.	CONJ.	IND.	CONJ.	IND.	CONJ.		
VR1	C. SUP.	48	-9330	---	2 ÁNG. 2"x1/4" (51mmx6.4mm)	12.34	---	---	---	---	1.25	---	48	---	38	1.3	-983
	C.INF.	48	9330	---	2 ÁNG. 2"x1/4" (51mmx6.4mm)	12.34	---	---	756	---	---	---	---	---	---	---	---
	ZIG-ZAG LATERAL	51	-1348	---	Ø 20	3.14	---	---	---	---	0.5	---	48	---	96	1.82	-781
	ZIG-ZAG SUP. E INF.	23	-618	---	Ø 12	1.13	---	---	---	---	0.3	---	21.3	---	71	1.42	-777
VR2	C. SUP.	32	-2570	---	2 ÁNG. 1"x1/8" (25mmx3.2mm)	3.02	---	---	---	---	0.75	---	31.9	---	43	1.16	-987
	C.INF.	32	2570	---	2 ÁNG. 1"x1/8" (25mmx3.2mm)	3.02	---	---	851	---	---	---	---	---	---	---	---
	ZIG-ZAG LATERAL	34	-578	---	Ø 12	1.13	---	---	---	---	0.3	---	31.9	---	106	2.02	-1033.2
	ZIG-ZAG SUP. E INF.	23	-277	---	Ø 8	0.5	---	---	---	---	0.2	---	21.3	---	107	2.05	-1135.7
VM1	MÉNSULA	200	170	137	C 220-80-2.5 DOBLE	20.6	1531	11	993	---	---	---	---	---	---	---	---

CERCHAS Y PERFILES DE ACERO - BLOQUE 4																	
DESIGN.	BARRA	L [cm]	N [kg]	M [tcm]	SECCION ADOPTADA	Ag [cm ²]	Jx [cm ⁴]	Y [cm]	σm.áx. [kg/cm ²]	r [cm]		Lkx [cm]		λx		σm.áx [kg/cm ²]	
										IND.	CONJ.	IND.	CONJ.	IND.	CONJ.		
VR1	C. SUP.	48	-9330	---	2 ANG. 2" x 1/4" (51mm x 6.4mm)	12.34	---	---	---	---	1.25	---	48	---	38	1.3	-983
	C. INF.	48	9330	---	2 ANG. 2" x 1/4" (51mm x 6.4mm)	12.34	---	---	756	---	---	---	---	---	---	---	---
	ZIG-ZAG LATERAL	51	-1348	---	Ø 20	3.14	---	---	---	---	0.5	---	48	---	96	1.82	-781
	ZIG-ZAG SUP. E INF.	23	-618	---	Ø 12	1.13	---	---	---	---	0.3	---	21.3	---	71	1.42	-777
VR2	C. SUP.	32	-2570	---	2 ANG. 1" x 1/8" (25mm x 3.2mm)	3.02	---	---	---	---	0.75	---	31.9	---	43	1.16	-987
	C. INF.	32	2570	---	2 ANG. 1" x 1/8" (25mm x 3.2mm)	3.02	---	---	851	---	---	---	---	---	---	---	---
	ZIG-ZAG LATERAL	34	-578	---	Ø 12	1.13	---	---	---	---	0.3	---	31.9	---	106	2.02	-1033.2
	ZIG-ZAG SUP. E INF.	23	-277	---	Ø 8	0.5	---	---	---	---	0.2	---	21.3	---	107	2.05	-1135.7
VR4	C. SUP.	48	-17200	---	2 ANG. 2 1/2" x 1/4" (64mm x 6.4mm)	15.74	---	---	---	---	1.21	---	48	---	40	1.14	-1246
	C. INF.	48	17200	---	2 ANG. 2 1/2" x 1/4" (64mm x 6.4mm)	15.74	---	---	1093	---	---	---	---	---	---	---	---
	ZIG-ZAG LATERAL	51	-1863	---	Ø 20	3.14	---	---	---	---	0.5	---	48	---	96	1.82	-1080
	ZIG-ZAG SUP. E INF.	23	-857	---	Ø 12	1.13	---	---	---	---	0.3	---	21.3	---	71	1.42	-1077